

HMR ARCHITECTS

2130 21st Street Sacramento, CA 95818 T 916 736 2724



DSA #02-123096

SHADE STRUCTURE

SOLANO COMMUNITY

4000 SUISUN VALLEY RD. FAIRFIELD, CA 94534

> DSA BACKCHECK

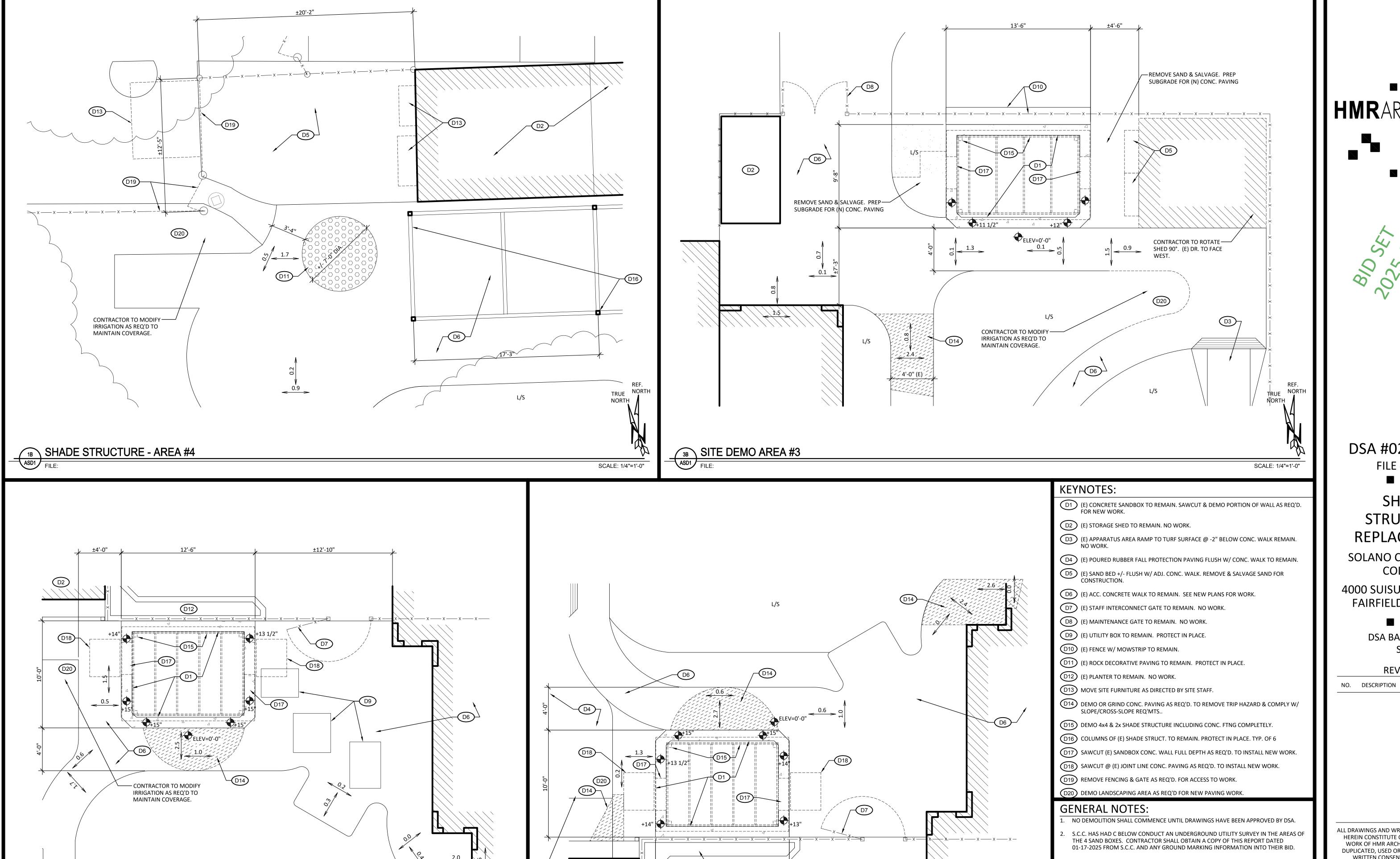
REVISIONS

ALL DRAWINGS AND WRITTEN MATERIAL APPEARING HEREIN CONSTITUTE ORIGINAL & UNPUBLISHED WORK OF HMR ARCHITECTS AND MAY NOT BE DUPLICATED, USED OR DISCLOSED WITHOUT THE WRITTEN CONSENT OF HMR ARCHITECTS

OVERALL SITE PLAN

MARCH 11, 2025

AS1



—CONTRACTOR TO MODIF

IRRIGATION AS REQ'D TO

SITE DEMO AREA #2

MAINTAIN COVERAGE.

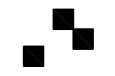
TRUE

SITE DEMO AREA #1

NORTH

SCALE: 1/4"=1'-0'

HMRARCHITECTS



2130 21st Street Sacramento, CA 95818 T 916 736 2724



DSA #02-123096

FILE #48-C1

SHADE STRUCTURE REPLACEMENT

SOLANO COMMUNITY COLLEGE

4000 SUISUN VALLEY RD.

FAIRFIELD, CA 94534

DSA BACKCHECK SET

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> SHADE STRUCTURE SITE DEMO

> > MARCH 11, 2025

CHECKED BY: KD JOB NO. 24055

LEGEND:

—×——×— CHAIN LINK FENCE TO REMAIN. NO WORK.

SURVEYED SLOPE OF GRADE IN PERCENT

(E) BUILDING

APPROXIMATE AREA OF CONC. DEMO.

ITEMS TO BE DEMOLISHED OR RELOCATED.

LANDSCAPING AREA TO REMAIN. PROTECT IN PLACE.

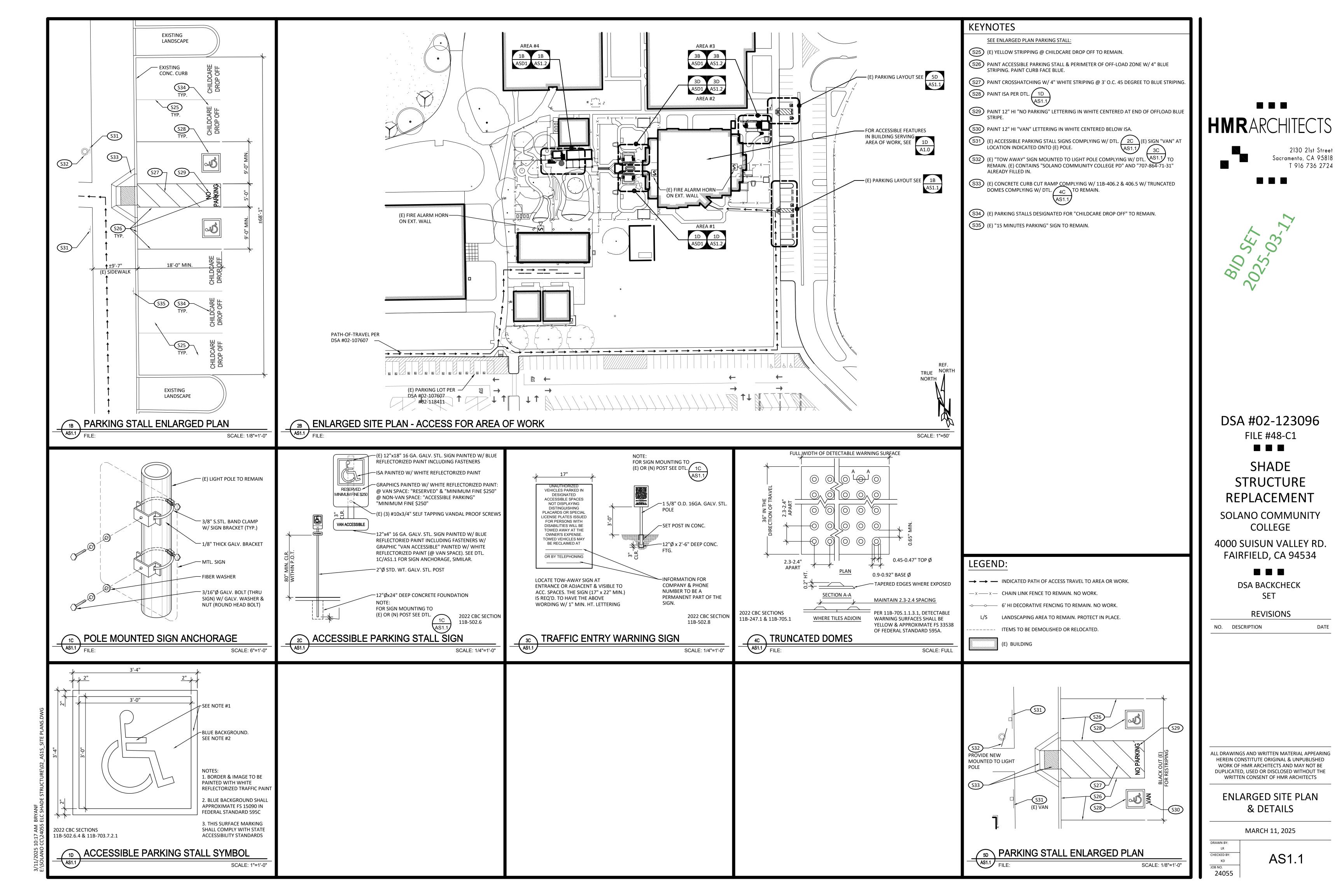
NORTH

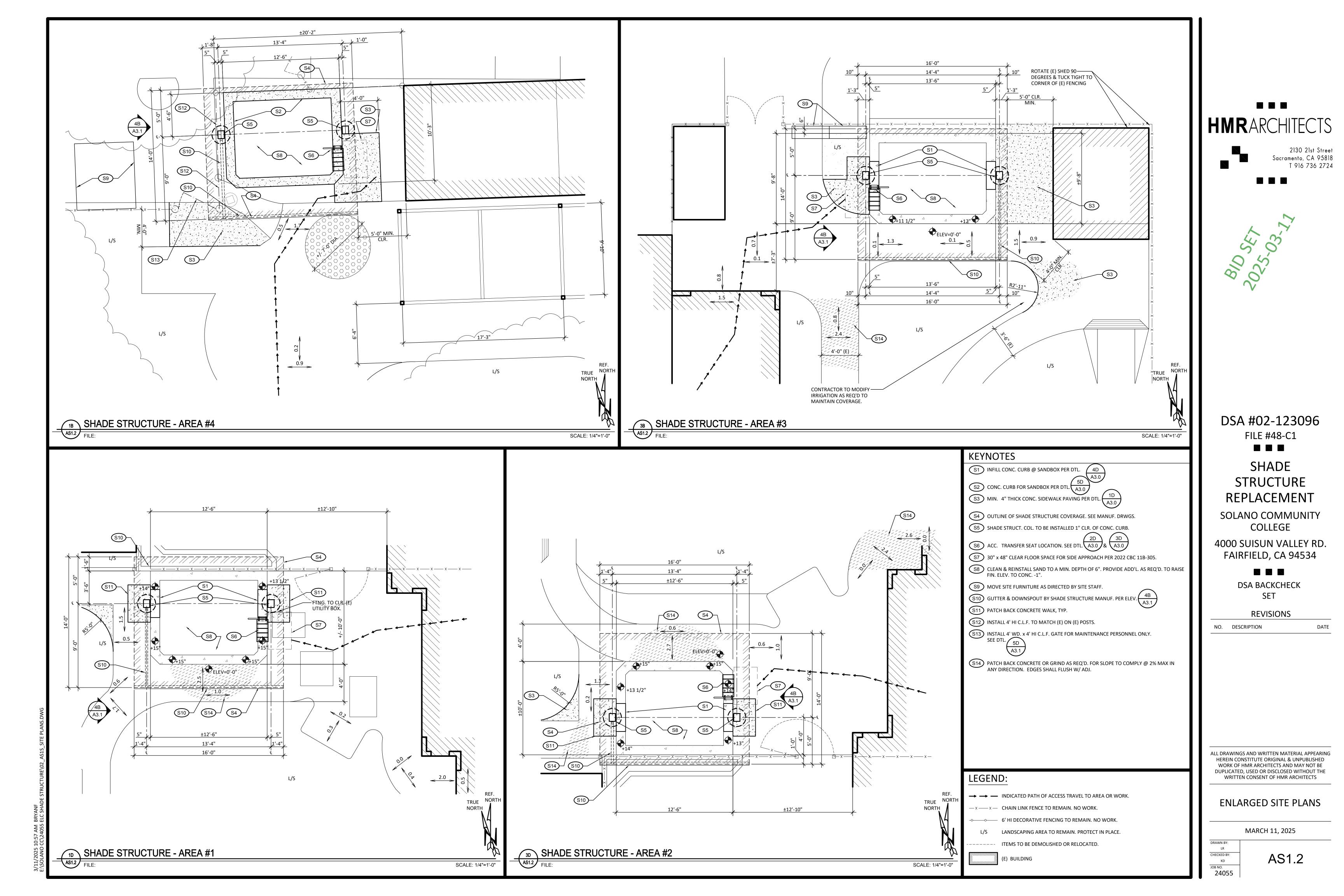
TRUE

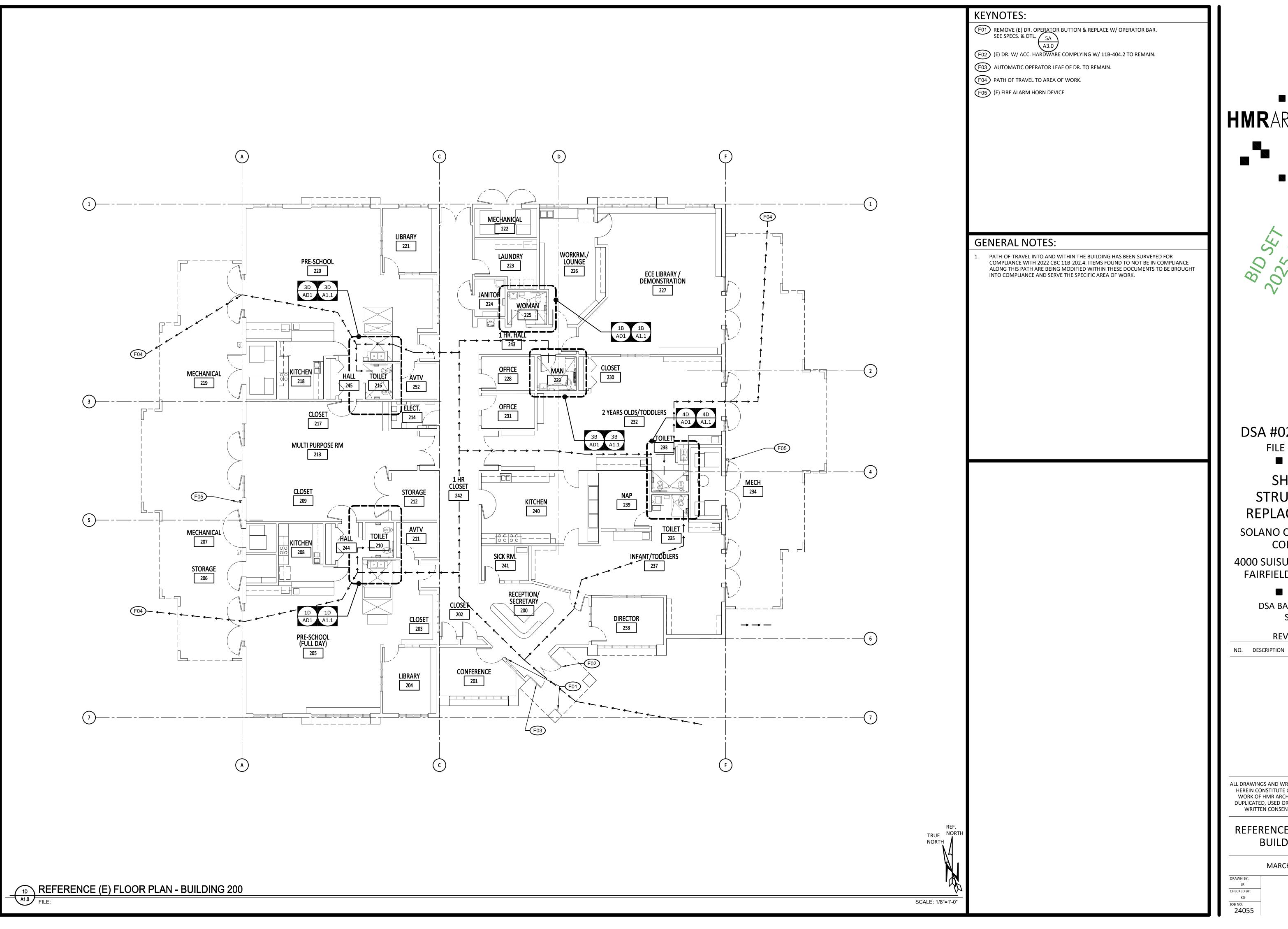
NORTH

SCALE: 1/4"=1'-0"

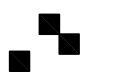
ASD1







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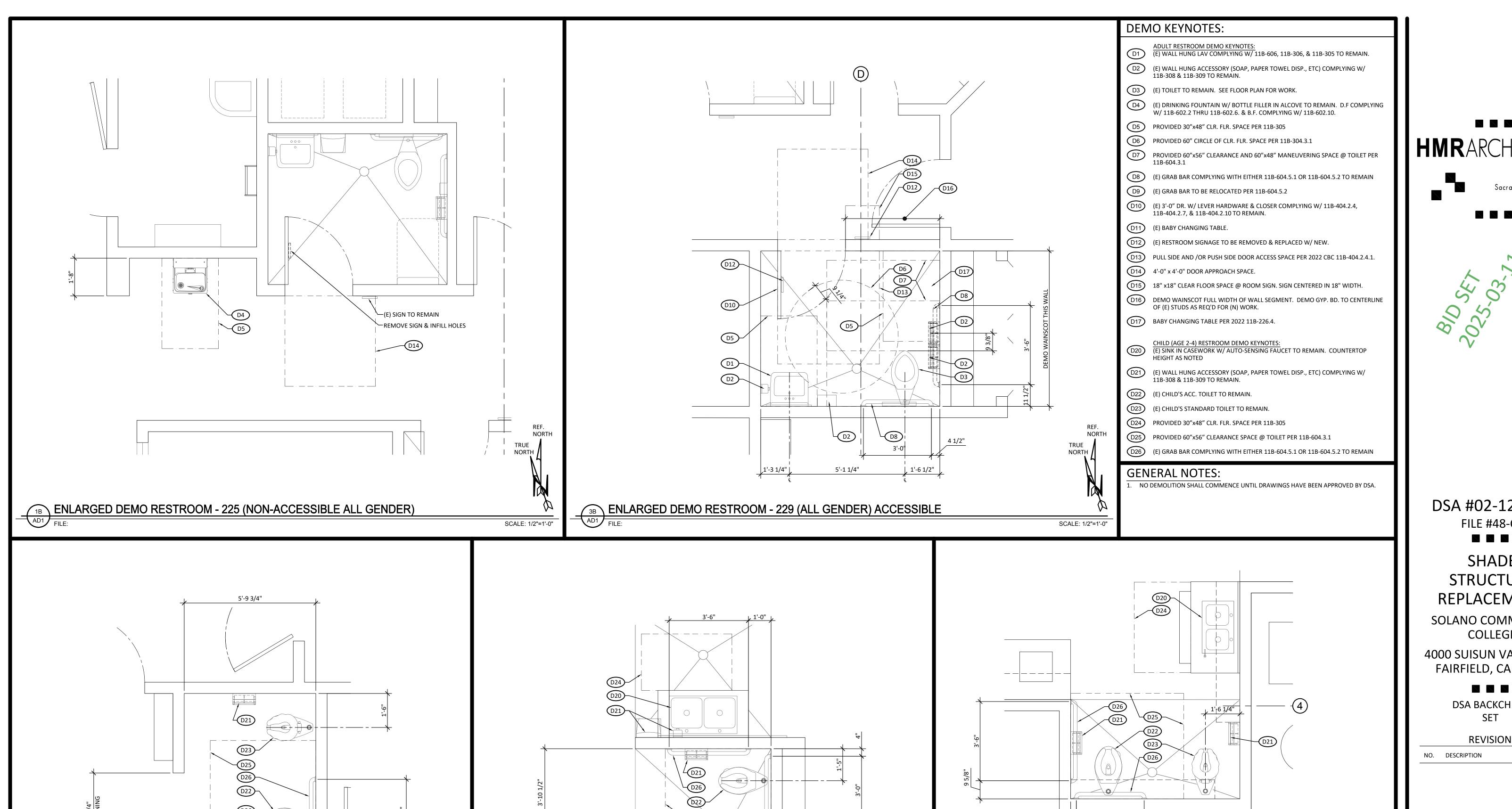
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REFERENCE FLOOR PLAN **BUILDING 200**

MARCH 11, 2025

A1.0



D23

TRUE NORTH

3

3D ENLARGED DEMO RESTROOM - 216

REF. NORTH

TRUE NORTH

SCALE: 1/2"=1'-0"

ENLARGED DEMO RESTROOM - 210

D25 NORTH NORTH TRUE NORTH ENLARGED DEMO RESTROOM - 233

AD1 FILE: SCALE: 1/2"=1'-0" SCALE: 1/2"=1'-0"

HMR ARCHITECTS



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> > SET **REVISIONS**

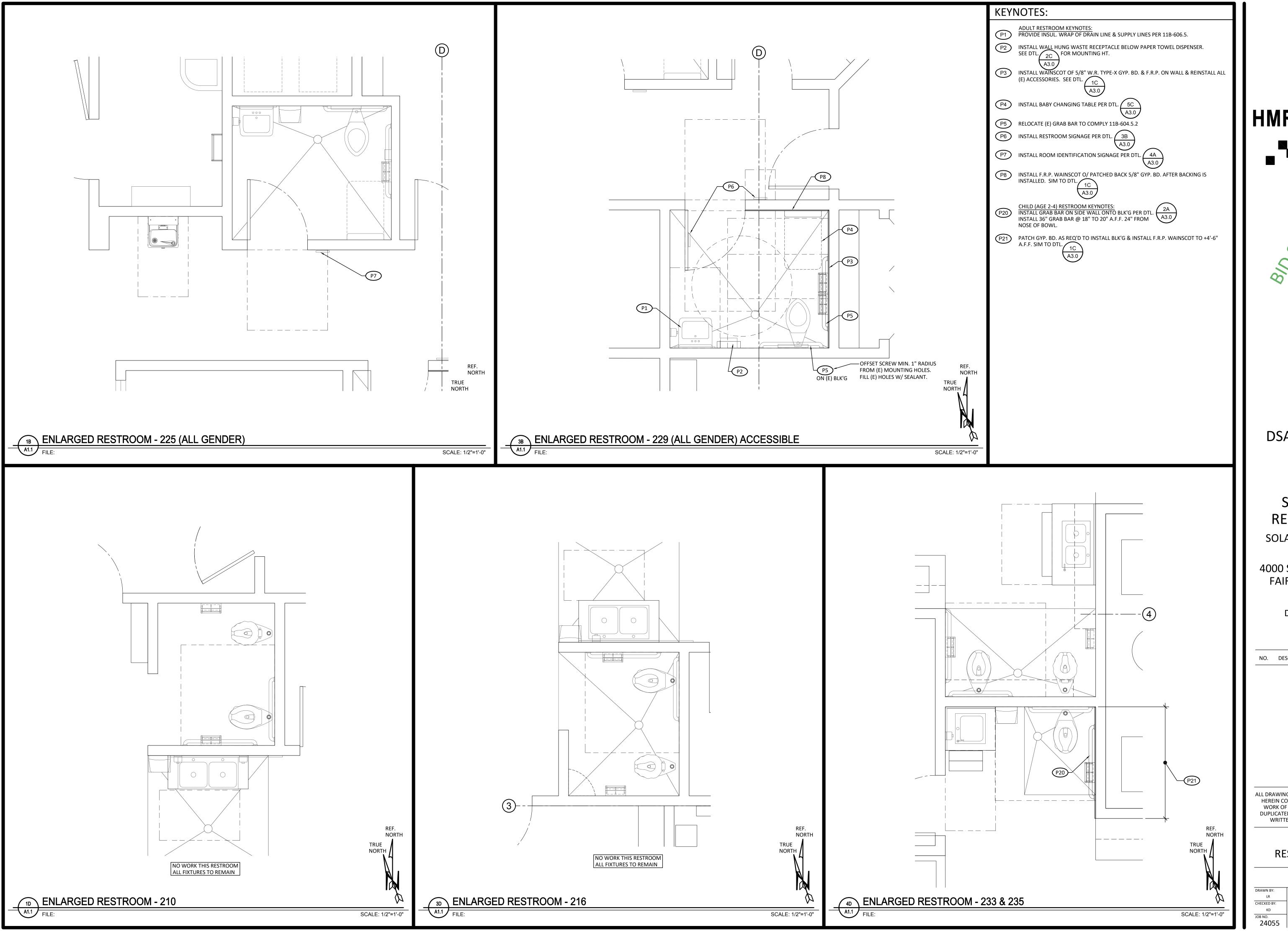
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> **ENLARGED DEMO RESTROOM PLANS**

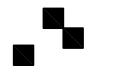
> > MARCH 11, 2025

CHECKED BY: JOB NO. 24055

AD1



HMRARCHITECTS



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> DSA BACKCHECK

SET

REVISIONS

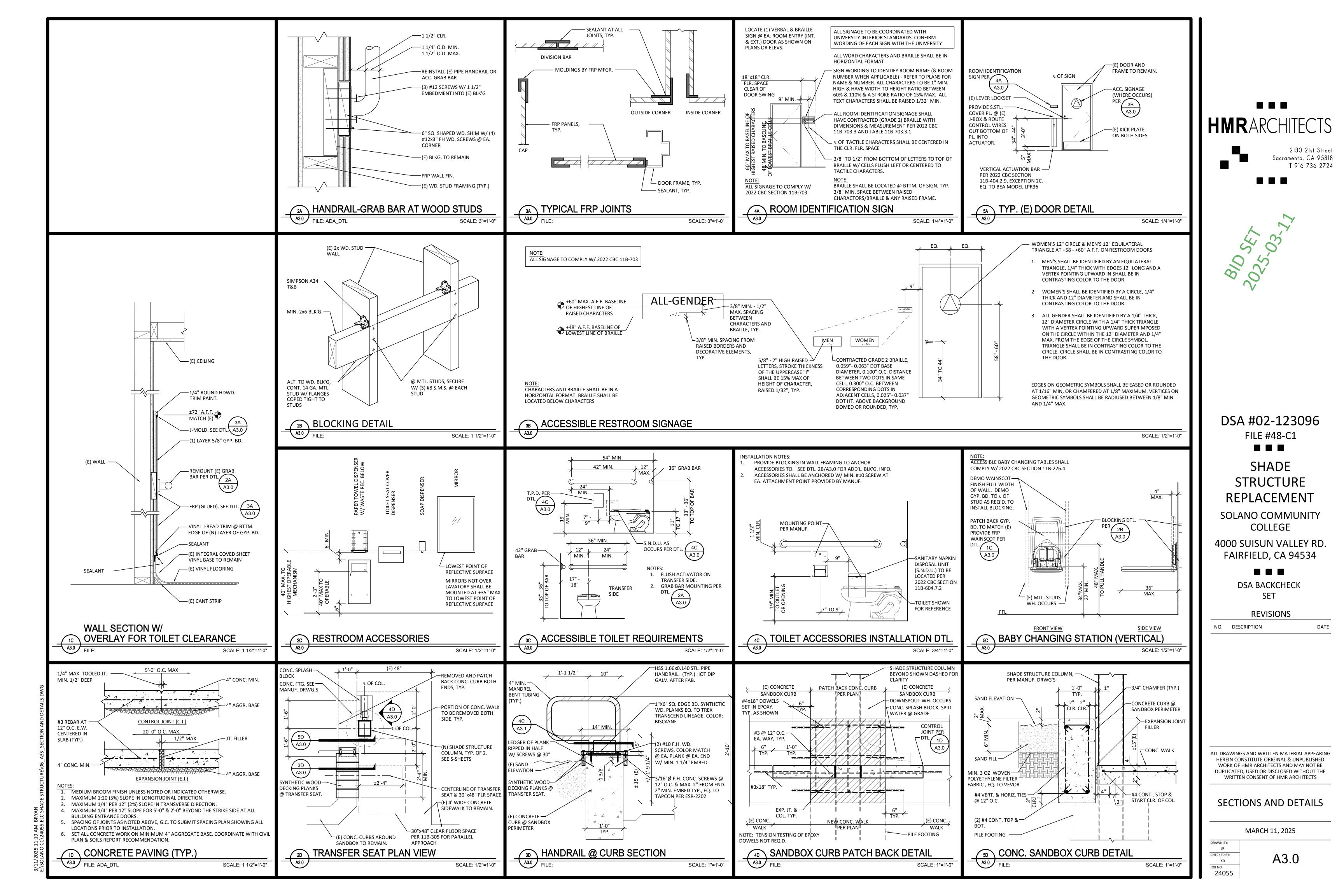
NO. DESCRIPTION

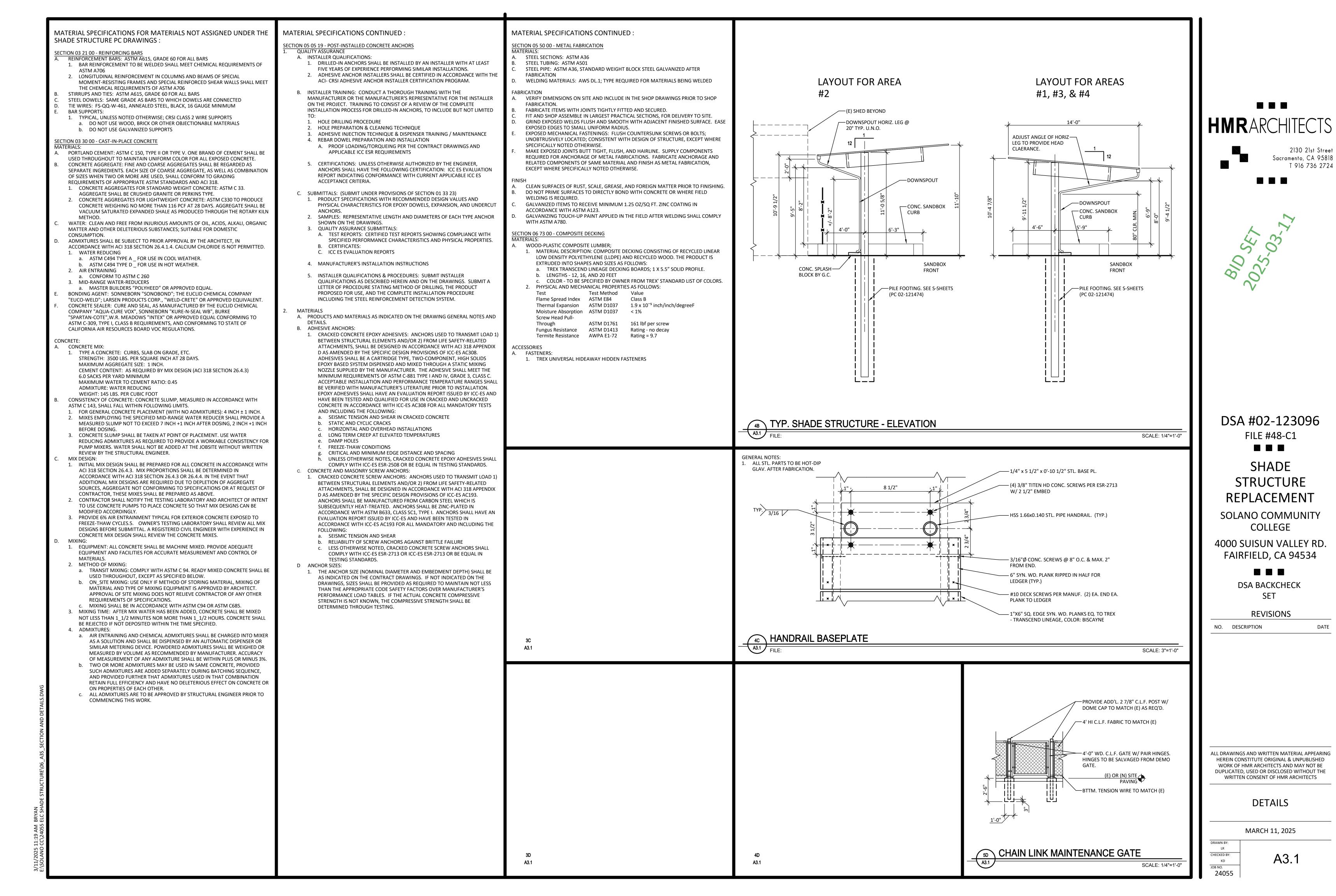
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> **ENLARGED RESTROOM PLANS**

MARCH 11, 2025

A1.1





GENERAL NOTES 1. ALL DIMENSIONS, CONDITIONS AND ELEVATIONS ARE TO BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORK OR FABRICATION. IF ANY DISCREPANCIES ARE FOUND OR IF ANY 2. THE CONTRACTOR WILL HOLD HARMLESS, INDEMNIFY AND DEFEND THE OWNER, THE ENGINEER, AND HIS CONSULTANTS, AND EACH OF THEIR OFFICERS AND EMPLOYEES AND AGENTS, FROM ANY AND ALL LIABILITY CLAIMS, LOSSES OR DAMAGE ARISING OR ALLEGED TO ARISE FROM THE PERFORMANCE OF THE WORK DESCRIBED HEREIN, BUT NOT INCLUDING THE SOLE NEGLIGENCE OF THE OWNER, THE ENGINEER 3. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE DESIGN INTENT. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. ALL WORK SHALL CONFORM TO THE LATEST APPLICABLE CONSTRUCTION SAFETY REQUIREMENTS OF O.S.H.A. AND ANY OTHER GOVERNMENTAL ENTITY HAVING JURISDICTION. 4. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON, OR NEAR THE CONSTRUCTION SITE. 5. ANY SUPPORT SERVICES PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVES DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER WHETHER OF MATERIAL OR WORK AND WHETHER PERFORMED PRIOR TO, DURING, OR AFTER COMPLETION OF CONSTRUCTION ARE PERFORMED SOLELY FOR THE PURPOSE OF ASSISTING IN QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH CONTRACT DRAWINGS AND SPECIFICATION, BUT THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION 6. ANY CHANGES TO THE APPROVED SET OF PLANS WITHOUT NOTIFYING THE ENGINEER PRIOR TO SUCH CHANGES ABSOLVES SAID ENGINEER FROM ANY AND ALL RESPONSIBILITY WITH RESPECT TO LIABILITY, DAMAGE OR EXTRA WORK RESULTING FROM SAID CHANGES. 7. NOTES AND DETAILS ON THE DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES. 8. THE TYPICAL DETAILS SHOWN ON THESE SHEETS SHALL APPLY IN ALL CASES UNLESS SPECIFICALLY SHOWN OTHERWISE. WHERE NO DETAIL IS SHOWN, CONSTRUCTION SHALL BE AS SHOWN FOR OTHER DESIGN LOADS: RISK CATEGORY II OR RISK CATEGORY III

ROOF LIVE LOAD: 20 PSF REDUCIBLE ROOF DEAD LOAD: 3 PSF

SNOW LOAD: Pg = Ps = 20 PSF & 30.5 PSF Ce = 1.1 Ct = 1.2 Cs = 1.0

Is = 1.0 RISK CATEGORY II Is = 1.1 RISK CATEGORY II ICE LOAD = 0 PSF SEISMIC: $I_E = 1.0$ AT RISK CATEGORY II $I_E = 1.25$ AT RISK CATEGORY III SEISMIC DESIGN CATEGORY = E ρ = 1.3 FOR OFFSET CONFIGURATION STRUCTURE IS A STEEL ORDINARY CANTILEVERED COLUMN SYSTEM (G2 PER ASCE7-16) R = 1.25/0.7 (FOR WORKING STRESS) Ω_0 = 1.25 Cd = 1.25 SOIL SITE CLASS = D FOR 12' COLUMN HEIGHT T = 0.361 FOR 14' COLUMN HEIGHT T = 0.45 FOR 16' COLUMN HEIGHT T = 0.497 BASE SHEAR: V = ρ x C8 x w RISK CATEGORY II V = 0.728W (ASD) FOR S_{DS} = 1.0 LOW SEISMIC & RC III V=0.728W (ASD) FOR S_{DS} = 0.8 RISK CATEGORY II V = 1.82W (ASD) FOR S_{DS} = 2.5 HIGH SEISMIC & RC III V=1.82W (ASD) FOR S_{DS} = 2.0 107 MPH, EXPOSURE C RISK CATEGORY II OR RISK CATEGORY III Kz = 0.90 Kzt = 1.0Ke = 1.00 WIND LOAD: qh = 21.42 (qh = 0.00256 x Kz x Ke x Kzt x Kd x V^2 x 0.6 (FOR WORKING STRESS)). THE STRUCTURE IS DESIGNED FOR CLEAR & OBSTRUCTED WIND FLOWS.

MAXIMUM BASIC WIND LOAD FOR PROJECT LOCATED IN SPECIAL WIND REGIONS SHALL BE EQUAL TO OR
LESS THAN 135 mph AND CONFORM WITH THE ADOPTED ORDINANCE OF THE CITY, COUNTY OR CITY AND
COUNTY IN WHICH THE PROJECT SITE IS LOCATED AND SHALL BE APPROVED BY DSA—SS. 11. ALLOWABLE SOIL BEARING IS BASED ON 1500 PSF & 100 PCFx2 PASSIVE PRESSURE PER CBC TABLE 1806A.2 & SECTION 1806A.3.4. SKIN FRICTION IS PER 1810A.3.3.1.4 & IS EQUAL TO 1500/6=250 PSF. FOR UPLIFT SKIN FRICTION SHALL BE 125 PSF (S.F. OF 2). 1/3 INCREASE WAS NOT USED FOR ANY VALUES. TOP OF PIER IS ASSUMED TO MOVE ONE-HALF INCH. 12. ALL WORK TO BE PERFORMED UNDER THE CONTINUOUS INSPECTION OF A D.S.A. APPROVED INSPECTOR. 13. ALL WORK SHALL CONFORM TO TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR). 14. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR A CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR AND THE ENGINEER. 15. A PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY THE OFFICE OF THE STATE ARCHITECT SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR. GENERAL NOTES NOTES FOR ACHITECT TO VERIFY & COMPLY GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY. 2. SUBSTITUTIONS AFFECTING DSA REGULATED ITEMS SHALL BE CONSIDERED AS A CONSTRUCTION CHANGE DOCUMENT OR ADDENDUM, AND SHALL BE APPROVED BY DSA PRIOR TO FABRICATION AND INSTALLATION PER DSA IR A-6 AND SECTION 338(C) PART 1, TITLE 24 CCR. 3. LOCATION OF ELECTRICAL COMPONENTS MOUNTED TO COLUMNS SHALL COMPLY WITH ASCE 24-14. 4. THE ARCHITECT SHALL VERIFY COMPLIANCE W/ 8B/S4 (IR PC-7, SECTION 5.8) IF COVER IS NEAR A SLOPE AND NOTIFY THE P.C. ENGINEER. IF COVER IS NEAR A SLOPE, THE INSTALLATION OF THIS STRUCTURE SHALL NOT BE SUBMITTED AS OVER-THE-COUNTER.

COLUMN, FILL COLUMN W CONCRETE TO TOP OF FOOTING, MUST FILL FROM TOP OF COLUMN (1.25 CU.FT. MIN.) CONCRETE OR AC PAVING REINFORCING, SEE COLUMN 4-L 2×2×3/6×3'-6" LONG \$ FOOTING TABLE 4H/52 ANGLES W 5-14-14 TEK SCREWS TO COLUMN

5CALE 1/-0"

OPTIONAL SPREAD FOOTING

@ ADDITIONAL COST

ALL MOLDS, ORNAMENTS, GROOVES, ETC. SHOWN ON THE ARCHITECTURAL DRAWINGS SHALL BE PROVIDED FOR IN THE FORM WORK BEFORE THE CONCRETE IS POURED.

2. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER EMBEDS SHALL BE IN PLACE AND SECURED TO FORM WORK PRIOR TO POURING OF CONCRETE.

3. REFER TO BOTH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF PLUMBING FIXTURES. 4. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE WALLS OR STRUCTURAL SLABS UNLESS SPECIFICALLY DETAILED.

5. CONSTRUCTION JOINTS NOT INDICATED ON THE DRAWINGS SHALL BE SO MADE AND LOCATED AS NOT TO IMPAIR THE STRENGTH OF THE STRUCTURE. PROVISION SHALL BE MADE FOR TRANSFER OF SHEAR AND OTHER FORCES THROUGHOUT THE JOINTS. THE CONTRACTOR SHALL OBTAIN THE ARCHITECTS APPROVAL OF CONSTRUCTION JOINT LOCATION IN ALL STRUCTURAL SLAB, BEAMS AND

6. SIDES OF FOOTINGS MAY BE POURED AGAINST STABLE EARTH.

7. THE QUALITY AND DESIGN OF CONCRETE SHALL COMPLY WITH TITLE 24 PART 2 EXCEPT ITEMS NOT SPECIFICALLY COVERED THEREIN SHALL CONFORM TO ACI 318.

8. ALL REINFORCING SHALL BE NEW STOCK DEFORMED BARS CONFORMING TO ASTM A615.

A. #4 BARS AND SMALLER...... GRADE 40 OR 60 B. #5 BARS AND LARGER.......GRADE 60 C. SEPARATE BARS 1-1/2 DIAMETERS CLEAR OR 1-1/2" CLEAR, WHICHEVER IS LARGER. 9. MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS:

CAST AGAINST EARTH (EXCEPT SLABS ON GRADE)..... SLABS ON GRADE.....EXPOSED TO EARTH OR WEATHER #5 BARS AND SMALLER.. 6 BARS AND LARGER NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS, WALLS, JOISTS 11 BARS AND SMALLER..

14 AND #18 BARS.... PRINCIPAL REINFORCING, TIES STIRRUPS, OR SPIRALS.....SHELLS AND FOLDED PLATE MEMBERS #5 BARS AND SMALLER. #6 BARS AND LARGER...

10. CONCRETE SHALL HAVE FOLLOWING MINIMUM REQUIRMENTS FOR THE FOLLOWING EXPOSURE FO- 3000 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.5. F2- 4500 PSI AT 28 DAYS & MAXIMUM WATER TO CEMENT RATIO OF 0.45. VERIFY COMPLIANCE

W/ ACI TABLE 19.3.3.1. 11. THE ENGINEER DOES NOT PROVIDE CONTRACT ADMINISTRATION FOR THE PROJECT, INCLUDING REVIEW OF CONCRETE MIXES.

12. PER ACI SECTION 19.3.2.1. FOOTING SHALL NOT BE EXPOSED TO FREEZING AND THAWING CYCLES, SHALL NOT BE EXPOSED TO WATER—SOLUBLE SULFATE IN SOIL BY PERCENT OF MASS 20.10%, SHALL NOT BE EXPOSED TO EXTERNAL SOURCE OF CHOLORIDES.

13. CEMENT SHALL COMPLY W/ ASTM C150 AND AGGREGATE SHALL COMPLY W/ ASTM C33.

ALL STRUCTURAL STEEL EXCEPT W SHAPES SHALL CONFORM TO ASTM A-36 AND SHALL BE FABRICATED AND ERECTED AS PER AISC SPECIFICATIONS FOR BUILDINGS. W SHAPES SHALL CONFORM

2. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE "B" AND STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500 GRADE "B", Fy=46KSI.

3. ALL LIGHT GAGE STEEL TO CONFORM TO ASTM A653 GRADE 55 FOR ALL STRUCTURAL SHAPES, A653 GRADE 33 FOR ALL BLOCKING, FLASHINGS, MISCELLANEOUS CONNECTION PLATES, AND ANGLES.

4. ALL UNFINISHED BOLTS SHALL BE ASTM A-307 UNLESS NOTED OTHERWISE.

5. USE AISC USUAL GAGES FOR BOLT HOLES IN ALL STEEL SECTIONS UNLESS OTHERWISE NOTED. 6. THE STEEL FABRICATOR SHALL PROVIDE ADEQUATE TEMPORARY BRACING FOR ERECTION.

. ALL BOLT HOLES ARE TO BE $^1\!\!/_6$ " OVERSIZED. ALL BOLTS SHALL HAVE WASHERS INSTALLED UNDER BOTH HEAD & NUT. SELF DRILLING SCREWS SHALL BE PER ESR-1976 OR EQUAL. 8. ALL STEEL SHALL BE PROTECTED FROM WEATHER AS FOLLOWS: STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED (MIMIMUN ASTM A123 OR A153, CLASS D) OR PAINTED WITH ZINC-RICH PRIMER, UNDERCOAT, AND FINISH COAT; OR EQUIVALENT PAINT SYSTEM. COLD-FORMED STEEL MEMBERS SHALL BE 55% ALUMINUM-ZINC ALLOY COATED PER ASTM A792/A792M STANDARD IN ACCORDANCE TO AISI S200 TABLE A4-1, CP 90 COATING DESIGNATION.

ALL EXPOSED STEEL FASTENERS, INCLUDING CAST-IN-PLACE ANCHOR BOLTS/RODS, SHALL BE STAINLESS STEEL (TYPE 304 MINIMUM), HOT-DIP GALVANIZED (ASTM A153, CLASS D MINIMUM), OR PROTECTED WITH CORROSION-PREVENTIVE COATING THAT DEMONSTRATED NO MORE THAN 2% OF RE RUST IN MINIMUM 1,000 HOURS OF EXPOSURE IN SALT SPRAY TEST PER ASTM B117. ZINC-PLATED FASTENERS DO NOT COMPLY WITH THIS REQUIREMENT. (EXAMPLE PROPRIETARY COATINGS THAT DO COMPLY WITH THE 1,000 HOUR REQUIREMENT INCLUDE BUT ARE NOT NECESSARILY LIMITED TO: QUIK GUARD BY SIMPSON, KWIK-COTE BY HILTI, STALGARD BY ELCO, VISTACORR BY SFS INTEC, ETC.)

GOVERNING CODES:

1. 2022 CALIFORNIA BUILDING STANDARDS ADMINISTRATIVE CODE (PART 1, TITLE 24, CCR).

2022 CALIFORNIA BUILDING CODE, VOLUMES 1 & 2 (PART 2, TITLE 24, CCR)

3. 2022 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR).

4. 2022 CALIFORNIA FIRE CODE (PART 9, TITLE 24, CCR).

1. COVERS ARE NOT DESIGNED TO BE ENCLOSED OR FOR STORAGE OF COMBUSTIBLE MATERIALS. 2. WALKWAY COVER HAS BEEN CHECKED FOR CLEAR & OBSTRUCTED WIND FLOW CONDITIONS & CAN BE

WITHIN 6" MIN. FROM AN EXISTING BUILDING. 3. WALKWAY PIER FOOTING HAS BEEN CHECKED FOR D.S.A. BULLETIN 09-06 REV..

4. ALL WORK SHALL COMPLY WITH C.F.C. CHAPTER 33 DURING CONSTRUCTION.

TESTING & INSPECTIONS REQUIRMENTS

1. INSPECTOR CLASS (MINIMUM REQUIREMENTS) CLASS 2

2. SELECTION OF THE PROJECT INSPECTOR AND TESTING AGENCY
BY THE SCHOOL DISTRICT AND APPROVED BY D.S.A., A/E OF RECORD AND STRUCTURAL ENGINEER 3. COST OF THE PROJECT INSPECTOR (CA ADMIN. CODE 4-333(B) AND TESTING AGENCY (CA ADMIN. CODE 4-335)
BY THE SCHOOL DISTRICT

4. COPIES OF THE REPORT TO ARCHITECT; STRUCTURAL ENGINEER; SCHOOL DISTRICT; D.S.A. (ORIGINAL); IOR; MANUFACTURER

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

PER TITLE 24, PART 1, SECTION 4-316 (D & E) OF THE CALIFORNIA CODE OF REGULATIONS, THE DISTRICT SHALL HIRE AN ARCHITECT OR STRUCTURAL ENGINEER TO BE IN GENERAL RESPONSIBLE CHARGE OF SITE SPECIFIC PROJECT.

2. FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE.

FOR SITE SPECIFIC PROJECT GERARD HOMER & ASSOCIATES RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR A PORTION OF THE PROJECT AS DESIGNED BY THE ARCHITECT FOR INCORPORATION INTO THE PROJECT.

STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM GERARD HOMER & ASSOCIATES RESPONSIBILITY FOR SITE SPECIFIC PROJECT.

SHADE STRUCTURE TESTING & INSPECTION GUIDELINE

THE DSA-103 GUIDELINES SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT-SPECIFIC FORM DSA-103. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING. DSA 103-22 LIST OF STRUCTURAL TESTS & SPECIAL INSPECTIONS - 2022 CBC

IMPORTANT: THIS FORM IS ONLY A SUMMARY LIST OF STRUCTURAL TESTS AND SOME OF THE SPECIAL INSPECTIONS REQUIRED FOR THE PROJECT. GENERALLY, THE STRUCTURAL TESTS AND INSPECTIONS NOTED ON THIS FORM ARE THOSE THAT WILL BE PERFORMED BY THE GEOTECHNICAL ENGINEER OF RECORD, LABORATORY OF RECORD, OR SPECIAL INSPECTIOR. THE ACTUAL COMPLETE TEST AND INSPECTION PROGRAM MUST BE PERFORMED AS DETAILED ON THE DSA APPROVED DOCUMENTS. THE APPENDIX AT THE BOTTOM OF THIS FORM IDENTIFIES WORK NOT SUBJECT TO DSA REQUIREMENTS FOR SPECIAL INSPECTION OR STRUCTURAL TESTING. THE PROJECT INSPECTOR IS RESPONSIBLE FOR PROVIDING INSPECTION OF ALL FACETS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO, SPECIAL INSPECTIONS NOT LISTED ON THIS FORM SUCH AS STRUCTURAL WOOD FRAMING, HIGH—LOAD WOOD DIAPHRAGMS, COLD—FORMED STEEL FRAMING, ANCHORAGE OF NON-STRUCTURAL COMPONENTS, ETC., PER TITLE 24, PART 2, CHAPTER 17A (2022 CBC). **NOTE: UNDEFINED SECTION & TABLE REFERENCES FOUND IN THIS DOCUMENT ARE FROM THE CBC, OR CALIFORNIA BUILDING CODE.

NOTE: REFERENCES ARE TO THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE (CBC) UNLESS OTHERWISE NOTED. TEST OR SPECIAL INSPECTION S1. GENERAL: SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF CONTROLLED FILL AND/OR EXCAVATIONS FOR FOUNDATIONS.

FOUNDATION EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. S4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS); G. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH PIER. b. VERIFY PIER LOCATIONS, DIAMETERS, PLUMBNESS & LENGTHS. RECORD CONCRETE OR GROUT VOLUMES. C1. CAST IN PLACE CONCRETE DURING CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, SLUMP & AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. d. TEST CONCRETE (COMPRESSION). f'c e. BATCH PLANT INSPECTION - PERIODIC, DESIGN COMPLIES WITH 1705A.3.3.1 ITEM 2. S/A1. STRUCTURAL STEEL, COLD-FORMED STEEL & ALUMINUM USED FOR STRUCTURAL PURPOSE

d. VERIFY IDENTIFICATION OF ALL MATERIALS AND:
 — MILL CERTIFICATES INDICATE MATERIAL PROPERTIES THAT COMPLY WITH REQUIREMENTS.
 — MATERIAL SIZES, TYPES AND GRADES COMPLY WITH REQUIREMENTS.

I. VERIFY AND DOCUMENT STEEL FABRICATION PER DSA-APPROVED CONSTRUCTION DOCUMENTS.

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING MAY BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF THE VERIFIED REPORTS FOR COMPLETED WORK. GERARD HOMER & ASSOCIATES SHALL NOT BE LISTED ON THE DSA 1 OR BE REQUIRED TO SIGN A DSA 1.DEL.

6. GERARD HOMER & ASSOCIATES WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTER OF THE PC WHICH ARISE DURING PLAN

MATERIAL VERIFICATION & TESTING:

EXP. 3-31-24 S2373

REVISIONS

NO. | DATE | BY | DESCRIPTION

PRE-CHECK (PC) DOCUMENT CODE: 2022 C.B.C. A SEPARATE PROJECT APPLICATION FOR CONSTRUCTION IS REQUIRED.

SHEET INDEX

SI FOUNDATION PLANS, GENERAL NOTES, DETAILS

52 ROOF FRAMING PLANS

53 SECTION, TYPICAL ELEVATION, DETAILS S4 SECTION, DETAIL

BUILDING DATA

A-2 FOR LUNCH SHELTER II-B CONSTRUCTION

ALLOW, AREA = 9500 SQ.FT.

HEIGHT = 8'-0" MIN. TO 16'-0" MAX. COLUMN HEIGHT

7. THE ARCHITECT OR DSA SHALL NOT ADD TO OR CHANGE THIS PC WITHOUT PERMISSION FROM GERARD HOMER & ASSOCIATES. SHEET INDEX & STRUCTURAL TESTS AND INSPECTIONS SHEET

SITE SPECIFIC INFORMATION TABLE SELECT OCCUPANT LOAD FACTOR (OLF) DETERMINATION OF BAY SPACING FOR COVER SEISMIC COEFFICIENT, Sa 1.514 NEXT TO RISK CATEGORY III & SDS > 2.0 ASSEMBLY-CONCENTRATED (7 SQ. FT./OCCUPANT) WIND SPEED (3 SEC. GUST) O MPI 93 0.6 SEISMIC COEFFICIENT, S1 (2) ASSEMBLY-NONCONCENTRATED (15 SQ. FT. WIND EXPOSURE CATEGORY 25' BAYS x 2.0 ACTUAL S_{DS} x 1.25 SEISMIC COEFFICIENT, Fa 1,2 /OCCUPANT) __ SPACING IN FEET ** 32 DETERMINE OCCUPANT LOAD - AREA/OLF 1.7 WIDTH OF COVER AREA IN FT. SEISMIC COEFFICIENT, FV ** BAY SPACING ON PLAN VIEWS TO BE REVISED TO MATCH & USE HIGHER SEISMIC COLUMNS & FOOTINGS. DETERMINE RISK CATEGORY 1.817 LENGTH OF COVERED AREA IN FT SEISMIC COEFFICIENT, Sms SEISMIC COEFFICIENT, Sm1 RISK CATEGORY II OCCUPANTS ≤ 300 1.275 224 AREA IN SQ. FT. NUMBER OF BAYS RISK CATEGORY III OCCUPANTS > 300 0.969 SITE SPECIFIC, Cs RISK CATEGORY III ADJACENT TO RISK 1.21 SEISMIC COEFFICIENT, Sds MAXIMUM COLUMN HEIGHT IN FT CATEGORY III BUILDING ROOF SLOPE IN./FT. NOTE: DUE TO REDUNDANCY FACTOR THERE CAN BE NO REDUCTION IN Spe PER ASCE 7-16 0.85 SEISMIC COEFFICIENT, Sd1 11112 12.8.1.3 FOR COVERS. TO FOR THE SITE CLASS D, Sm1 SHALL BE INCREASED BY 50% UNLESS A SITE SPECIFIC LIQUEFIABLE SOIL OR SITE CLASS F & E GEOHAZARD REPORTS GROUND MOTION HAZARD ANALYSIS IS PERFORMED. Sms SHALL NOT BE LESS NOT REQUIRED GEOHAZARD REPORT REQUIRED STRUCTURE IS LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS GEOHAZARD REPORTS ARE NOT REQUIRED FOR CANTILEVERED COLUMN OPEN STRUCTURES PROVIDED THEY ARE CONSTRUCTED OF METAL, DO NOT EXCEED 4,000 SF. IN PLAN AREA AND ARE NOT LOCATED WITHIN STATE OR LOCAL Property of the state of the st & E, OVER-THE-COUNTER SUBMITTAL IS NOT ALLOWED AND REGULAR PROJECT SUBMITTAL IS REQUIRED. IF SITE IS NOT IN A MAPPED LIQUEFACTION HAZARD ZONE, IT MAY BE PRESUMED THAT NO LIQUEFACTION HAZARD EXISTS ON THAT SITE UNLESS A SITE—SPECIFIC GEOTECHNICAL REPORT IDENTIFIES THAN OR EQUAL TO Ts, EQUATION 12.8-2 GEOHAZARD ZONES. THE STRUCTURES MAY BE SPLIT INTO MULTIPLE SHALL BE USED. SEISMICALLY SEPARATED STRUCTURES TO STAY BELOW THE 4,000 SF. TRIGGER. SUCH HAZARD. ③ PER ASCE 7-16 EQUATION 12.8-2. CONCRETE EXPOSURE F2* CONCRETE EXPOSURE FO

* F2 IS REQUIRED FOR ANY SNOW LOAD 30 PSF OR GREATER.

IF SITE SPECIFIC PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN X, A LETTER STAMPED AND SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ON THIS P.C., SEE GENERAL NOTES NUMBER II.

SINGLE POLE WALKWAY OPTIONS TABLE														
LOADING		COLUMN HEIGHT		ROOF PANEL OPTION				PRO	OJECTION NOT		:s. 1.	SEE 6H/53 FOR ROOF DECK PROFILES & SECTION PROPERTIES.		
X	20 PSF	X	12' HEIGHT	X	McELRC	Y M	EGA-RIB		8'-0"		3.	. LIVE LOAD, LL = ROOF LIVE LOAD. . SNOW LOAD, SL = SNOW LOAD.		
	20 PSF SL	<u> </u>	14' HEIGHT		AEP S	PAN	HR-36		10'-0"		4.	3 P.S.F. MISC. DEAD LOAD NOT ALLOWED WITH SNOW LOAD. MISC. DEAD LOAD CAN INCLUDE SPRINKLERS OR SOLAR. BOTH ARE NOT ELIGIBLE FOR OVER THE COUNTER PLAN CHECK.		
	30 PSF SL		16' HEIGHT				3 P.S.F. ⁴		12'-0"			LOADS CAN NOT BE SUPPORTED BY ROOF PANELS, ONLY THE BEAMS & PURLING CAN SUPPORT MISC, DEAD LOAD,		
RC	OF PITCH		COLUMN LOCATION		SEISMIC	ı	MISC. DEAD LOAD	X	14'-0"		5.	SEE SITE SPECIFIC TABLE.		
X	2:12 MAX.		OFF-SET		LOWER *		YES			•		CONFIGURATION PER 6K/S4		
	412 MAX.		CENTERED	X	HIGHER **	X	NO		"A*			"B" "C" "D"		
<u> </u>					rc III s _{ds} >	2.0					6.	FOR SNOW LOAD: IF THE HORIZONTAL SEPARATION DISTANCE BETWEEN AD JACENT STRUCTURES, 8, IS LESS THAN		

* R.C. II Sds ≤ 1.0 & R.C. III Sds ≤ 0.80

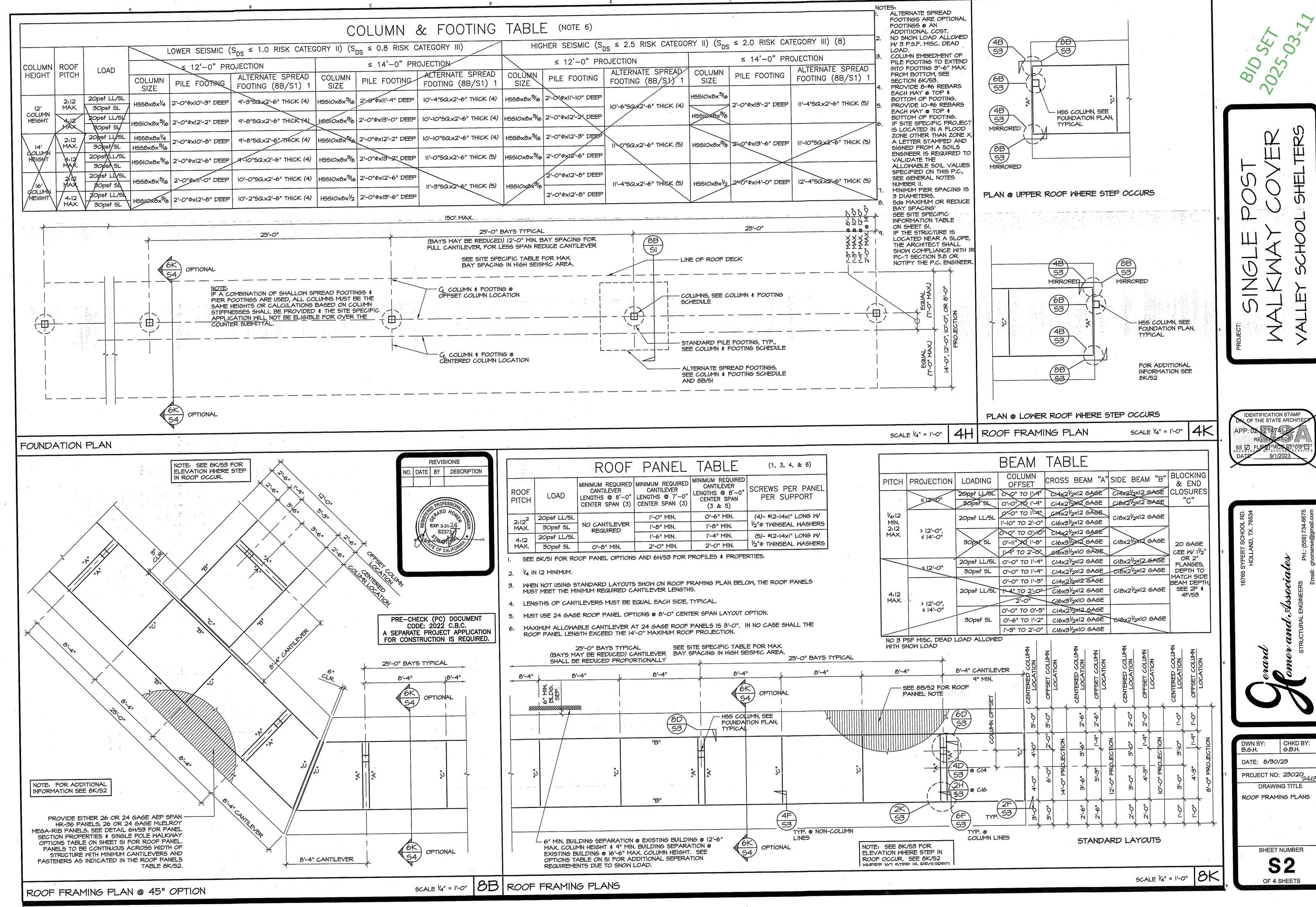
** R.C. || Sds ≤ 2.5 & R.C. ||| Sds ≤ 2.0

20 FT. (6.1 M) AND LESS THAN SIX TIMES THE VERTICAL SEPARATION DISTANCE (9(6h), THEN THE REQUIREMENTS FOR THE LEEWARD DRIFT OF SECTION 7.7.1 SHALL BE USED TO DETERMINE THE DRIFT LOAD ON THE LOWER STRUCTURE. II PROVIDED BY THE PC APPLICANT & THE SITE APPLICATION IS NOT ELIGIBLE FOR OVER THE COUNTER SUBMITTAL.

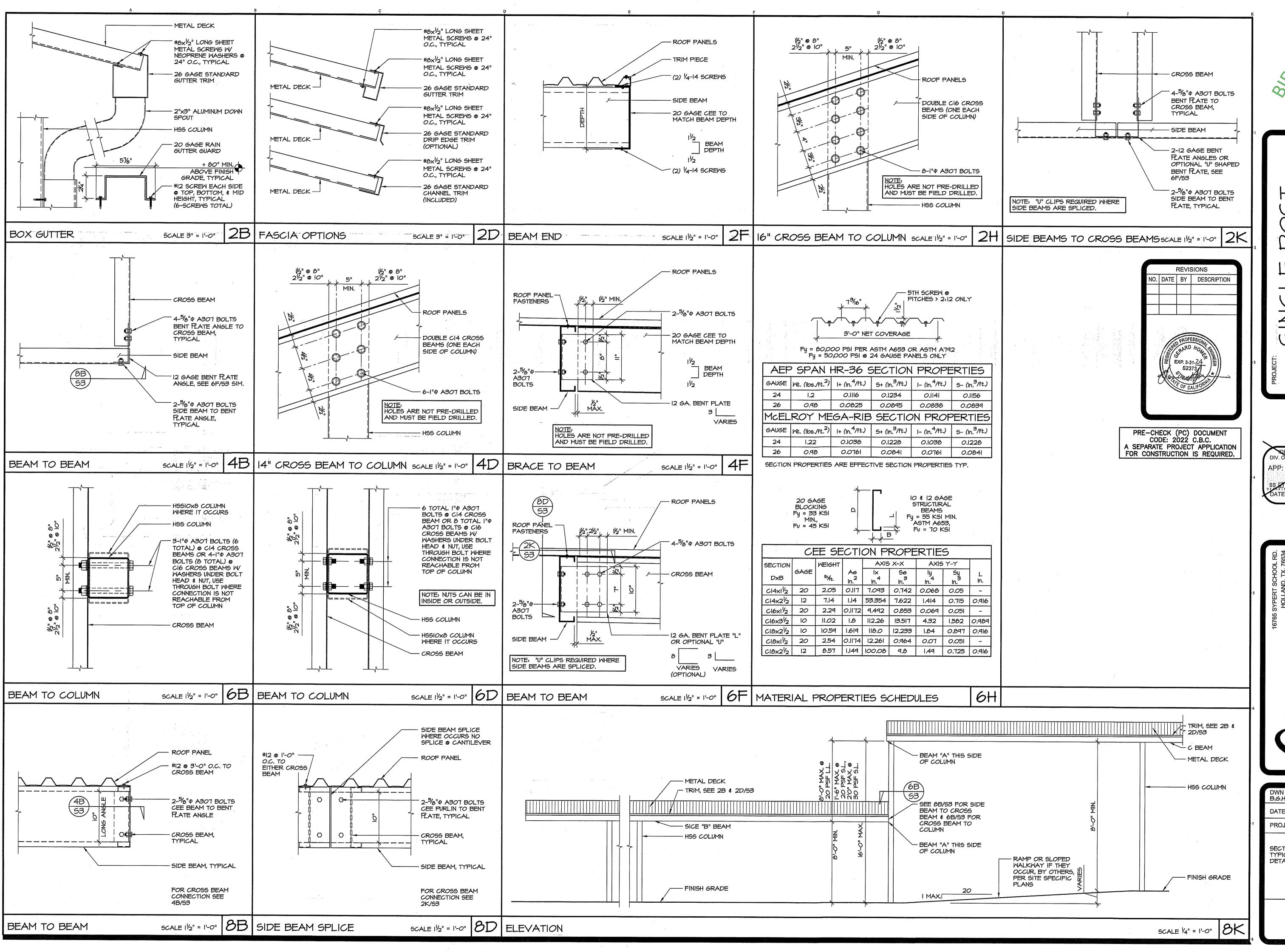
F THE STATE ARCH

DWN BY: B.G.H.	CHKD BY: 6.B.H.
DATE: 8/30/	23
PROJECT NO	23020 45049 24
	IG TITLE
FOUNDATION GENERAL NO DETAILS	
SHEET	NUMBER
S	
OF 4 S	SHEETS

OPTIONS TABLE & SITE SPECIFIC INFORMATION TABLE



PROJECT NO: 23020



810.5k7 20.5k7 20.5k7

SINGLE POST MALKMAY COVER

DENTIFICATION STAMP
DIV. OP THE STATE ADCHITECT

APP: 02-12 14 4 PC

BEMEWEDITOR

SS OF FLS ADMACSE PROSHED TO BE ARTMENT OF GENERAL SERVICES
DATE: 9/1/2023



DWN BY: B.G.H. CHKD BY: G.B.H.

DATE: 8/30/23

PROJECT NO: 23020

CALGA

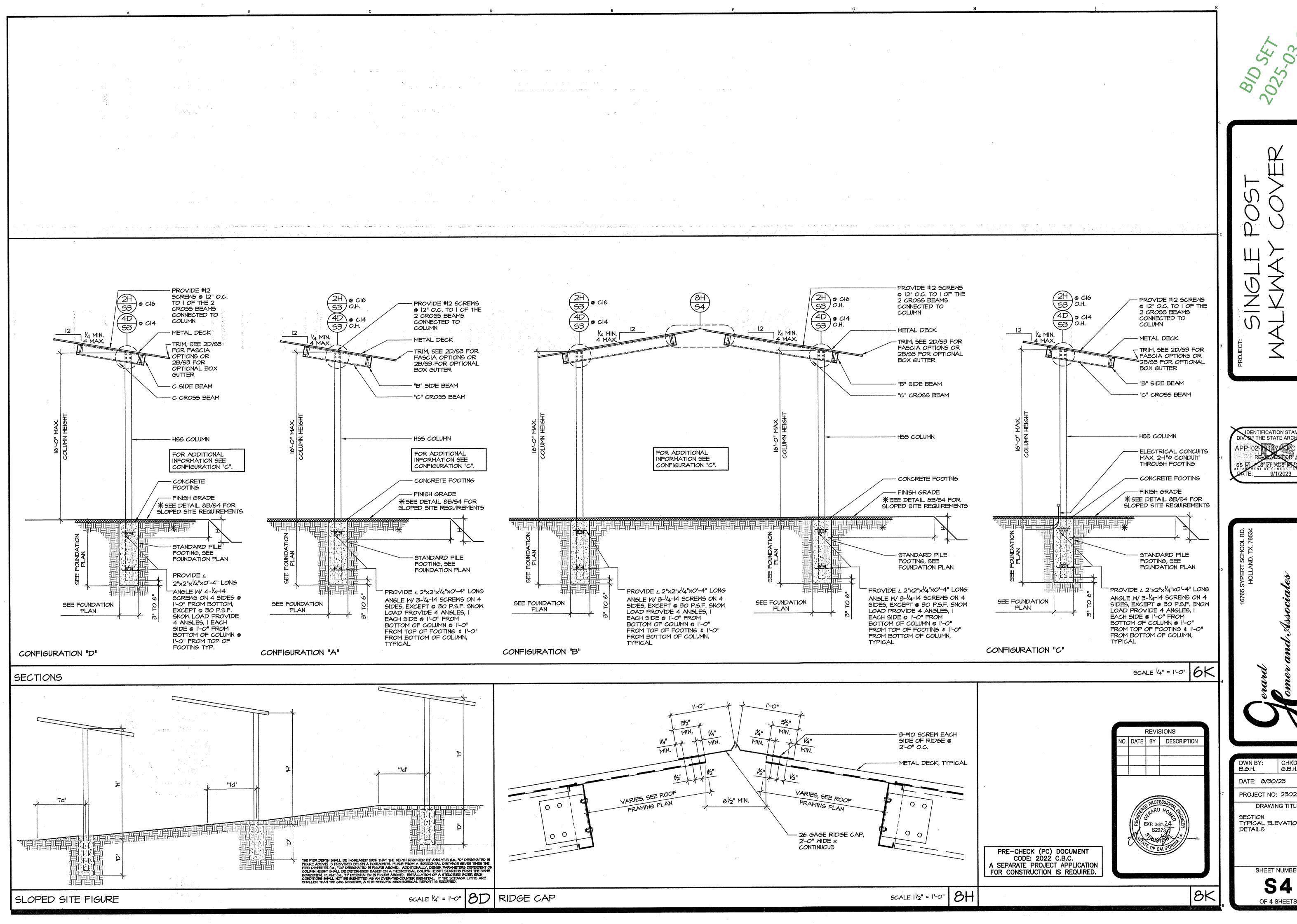
DRAWING TITLE

SECTION
TYPICAL ELEVATION
DETAILS

SHEET NUMBER

S 3

OF 4 SHEETS





DATE: 8/30/23 PROJECT NO: 23020 DRAWING TITLE TYPICAL ELEVATION SHEET NUMBER